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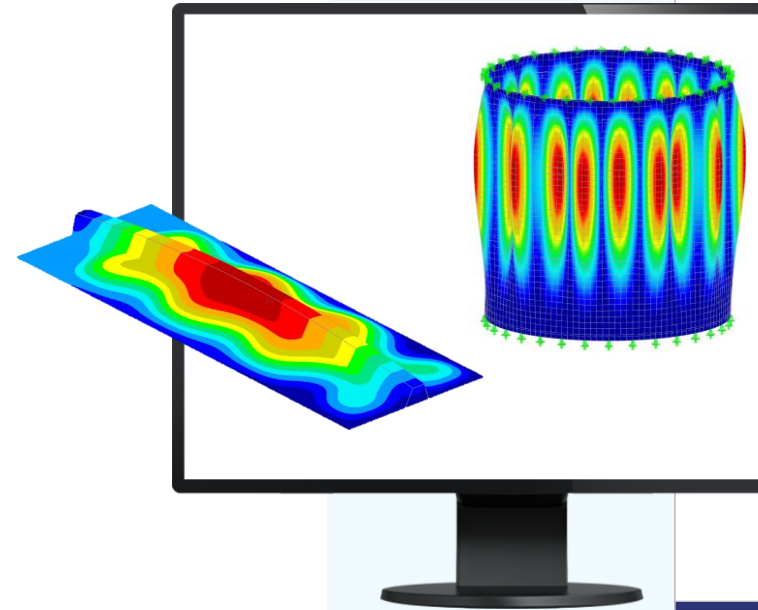


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Webinar

Buckling Analysis in RFEM 6



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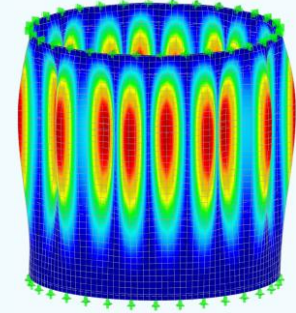
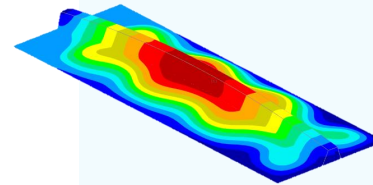
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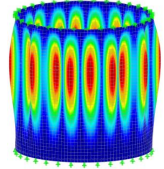
CONTENT

- 01 Analysis for shell buckling using global MNA and LBA calculations according to EN 1993-1-6
- 02 Analysis for plate buckling using GMNIA method according to EN 1993-1-5





Buckling Analyses of Steel Shell Structures acc. to EN1993-1-6



Stress-based buckling analysis

- Simple application for expert engineers
- Low requirements for computer technology (often hand calculation formulas used)
- Economic results difficult to achieve for load situations significantly differing from conventional buckling shapes

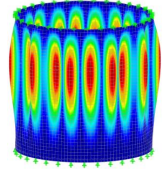
Design by global numerical MNA/LBA analysis

- More background knowledge for shell stability required
- Higher requirements for computer technology (materially nonlinear analysis (MNA), linear elastic bifurcation analysis (LBA))
- Computer technology using FE analysis consequently applied

Design by global numerical GMNIA analysis

- Excellent background knowledge for shell stability required (e.g. correct application of imperfections (pre-forming) is complex)
- Considerable requirements for computer technology

Example: Shell Buckling Design by Global Numerical MNA/LBA Analysis acc. to [3]



Technical data

Liquid: $\gamma = 10 \text{ kN/m}^3$

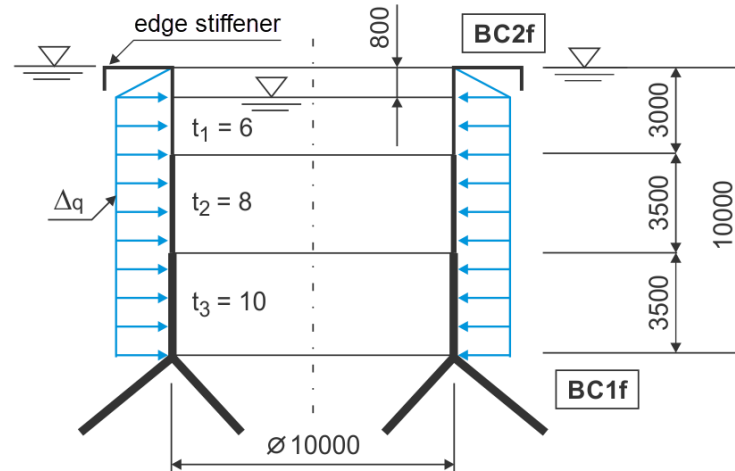
Material: S 235

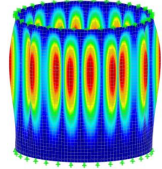
Manufacturer quality: class A

Load (1.0 x differential pressure)

$\Delta q_d = 8.0 \text{ kN/m}^2$

System





Analysis

Elastic critical buckling resistance ratio

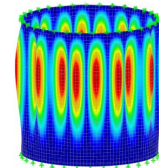
$$r_{Rcr} = 1.504 \text{ (FE eigenvalue analysis (LBA) in RFEM)}$$

Plastic reference resistance ratio ([2], Eq. 8.24)

$$r_{Rpl} = t \cdot f_{yk} / \sqrt{n_{x,Ed}^2 - n_{x,Ed}n_{\theta,Ed} + n_{\theta,Ed}^2 + 3n_{x\theta,Ed}^2}$$

The lowest value of plastic resistance ratio so calculated should be taken as the estimate of the plastic reference resistance ratio r_{Rpl} .

NOTE: A safe estimate of r_{Rpl} can usually be obtained by applying expression (8.24) in turn at the three points in the shell where each of the three buckling-relevant membrane stress resultants attains its highest value, and using the lowest of these three estimates as the relevant value for r_{Rpl} . [2]



Analysis

$r_{Rpl} = 35.6$ (materially non-linear analysis (MNA in RFEM))

Overall relative slenderness ([2], Eq. 8.25)

$$\bar{\lambda}_{ov} = \sqrt{r_{Rpl}/r_{Rcr}}$$

$$\bar{\lambda}_{ov} = \sqrt{35.6/1.504}$$

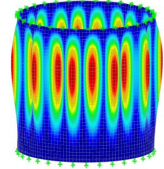
$$\bar{\lambda}_{ov} = 4.87$$

Circumferential elastic imperfection reduction factor ([2], Tab. D.5)

$$\alpha_{ov} = \alpha_{\theta} = 0.75$$

Plastic range factor ([2], D.26)

$$\beta = 0.60$$



Analysis

Plastic limit relative slenderness ([2], Eq. 8.16)

$$\bar{\lambda}_p = \sqrt{\alpha/(1 - \beta)}$$

$$\bar{\lambda}_p = \sqrt{0.75/0.40}$$

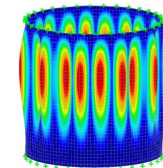
$$\bar{\lambda}_p = 1.37 \ll 4.87 \rightarrow \text{pure elastic buckling}$$

Buckling reduction factor ([2], Eq. 8.15)

$$\chi_{ov} = \frac{\alpha}{\bar{\lambda}^2}$$

$$\chi_{ov} = 0.75/4.87^2$$

$$\chi_{ov} = 0.0316$$



Analysis

Characteristic buckling resistance ratio ([2], Eq. 8.26)

$$r_{Rk} = \chi_{ov} \cdot r_{Rpl}$$

$$r_{Rk} = 0.0316 \cdot 35.6$$

$$r_{Rk} = 1.125$$

Design buckling resistance ratio ([2], Eq. 8.27)

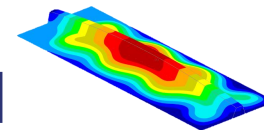
$$r_{Rd} = r_{Rk} / \gamma_{M1}$$

$$r_{Rd} = 1.125 / 1.1$$

$$r_{Rd} = 1.02 > 1 \rightarrow \text{design fulfilled}$$

[→ Another example available in Knowledge Base](#)

Example: (Buckling) Verification of a stiffened Plate using GMNIA



Plate

Material: S 355

Thickness: $t = 14 \text{ mm}$

Plate loading

$\sigma_1 = \sigma_2 = 21,0 \text{ kN/cm}^2$

$\tau = 1,0 \text{ kN/cm}^2$

Trapezoidal Stiffener

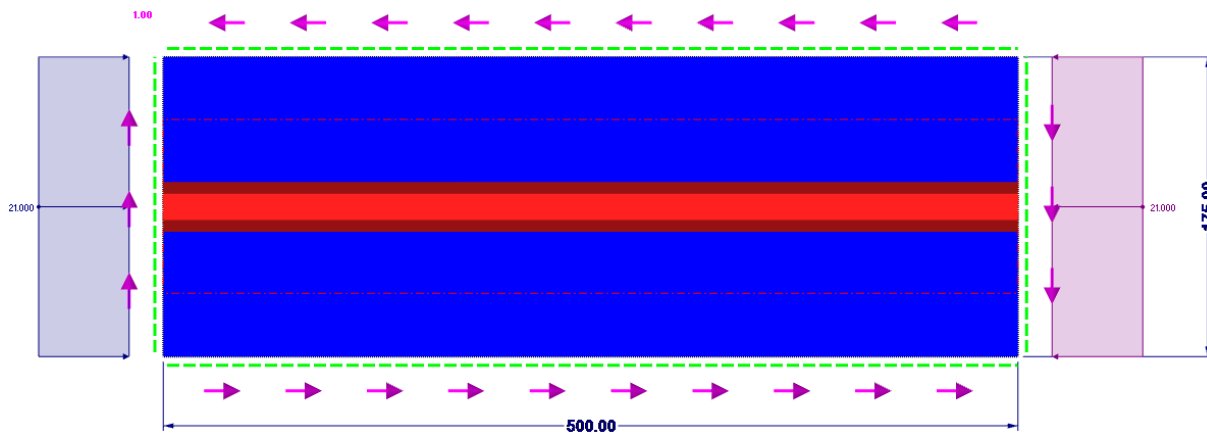
Material: S 355

$h = 200 \text{ mm}$

$b_v/b_o = 300 \text{ mm} / 150 \text{ mm}$

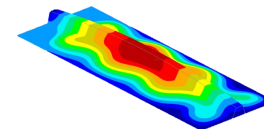
$t = 6 \text{ mm}$

System

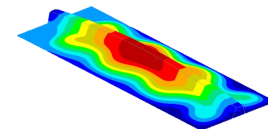




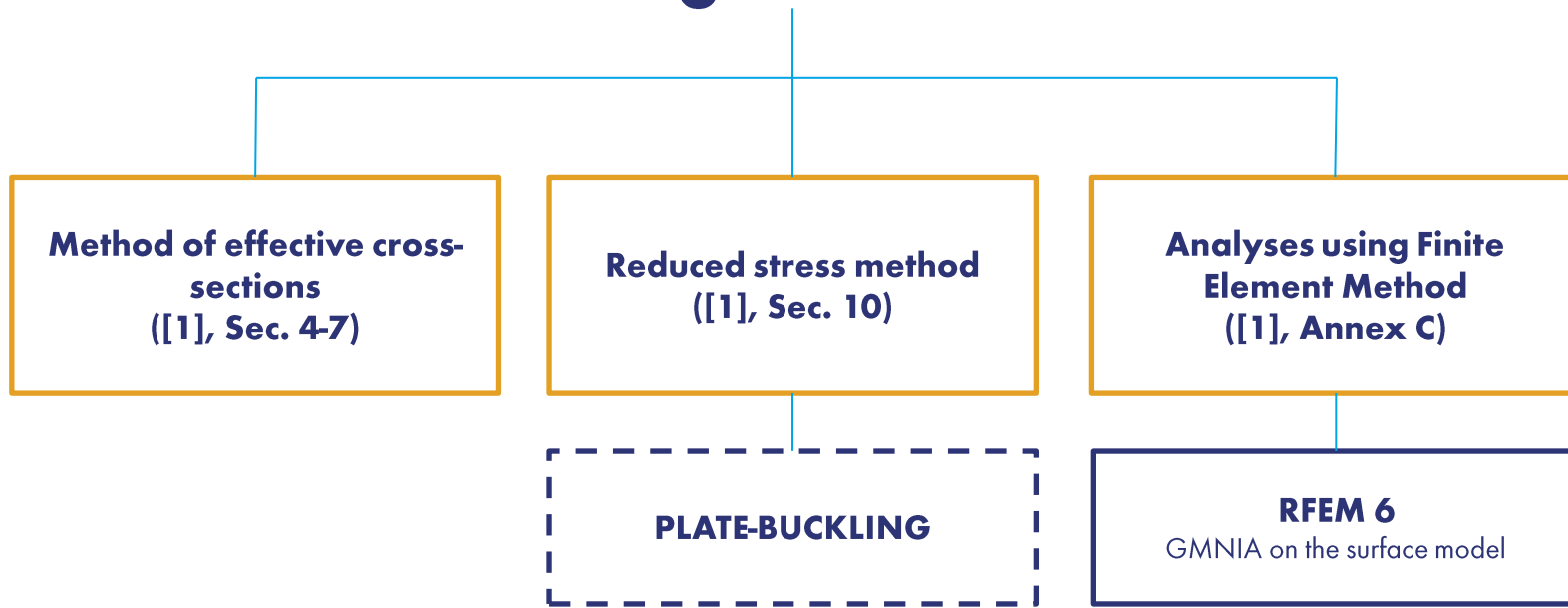
What are the characteristics of a GMNIA?



Analysis type	Deformations	Material	Geometry
Linear elastic analysis (LA)	linear	linear elastic	perfect
Linear bifurcation (eigenvalue) analysis (LBA)	eigenshape	linear elastic	perfect
Materially non-linear analysis (MNA)	linear	elastic-plastic	perfect
Geometrically non-linear analysis (GNA)	nonlinear	linear elastic	perfect
Geometrically and materially non-linear analysis (GMNA)	nonlinear	nonlinear	perfect
Geometrically non-linear elastic analysis with imperfections (GNIA)	nonlinear	linear elastic	imperfect
Geometrically and materially non-linear analysis with imperfections (GMNIA)	nonlinear	nonlinear	imperfect

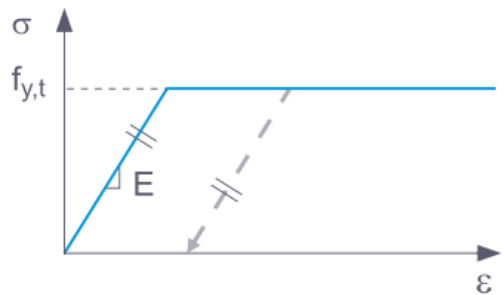
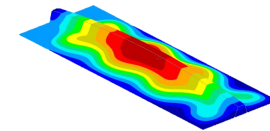


Buckling Analyses of Steel Plates According to EN1993-1-5

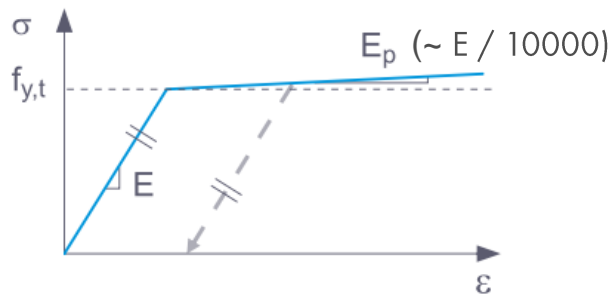




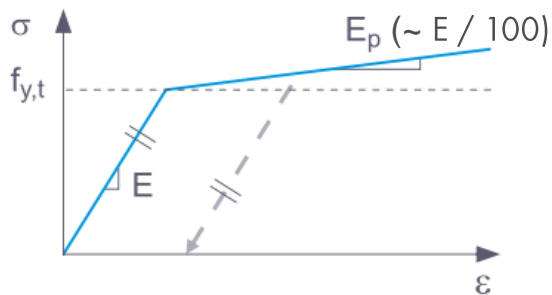
Nonlinear Material Models



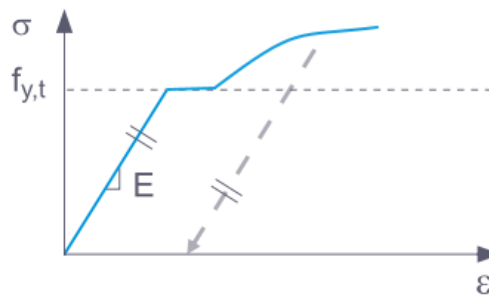
a) linear elastic - perfectly plastic



b) linear elastic - perfectly plastic with pseudo-hardening



c) linear elastic - linear hardening

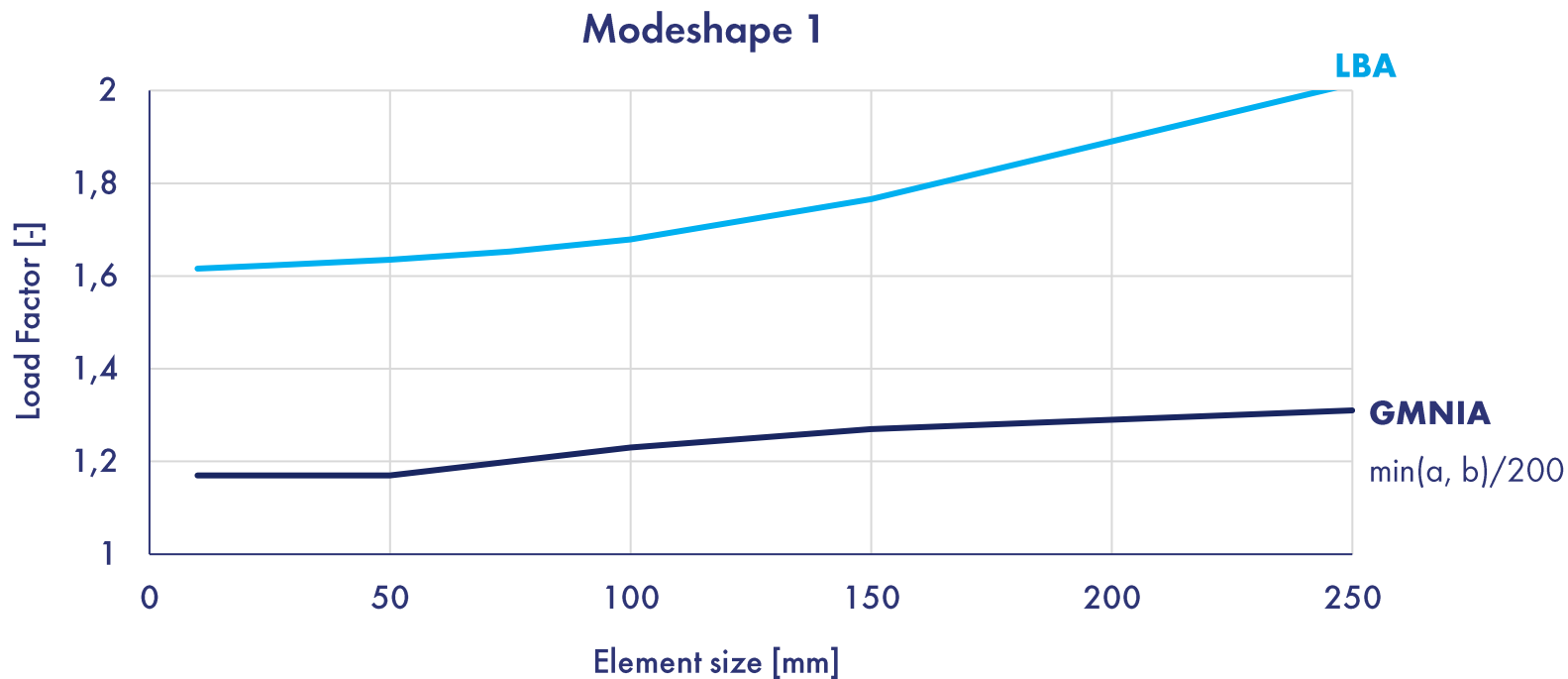
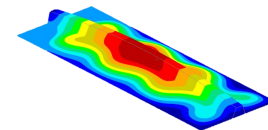


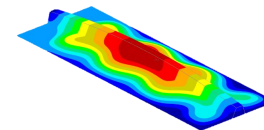
d) true stress-strain curve





Mesh convergence study

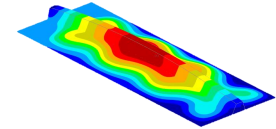




Imperfections

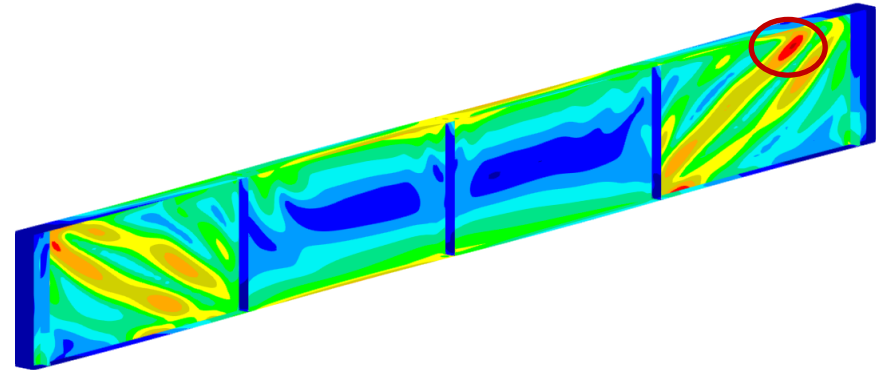
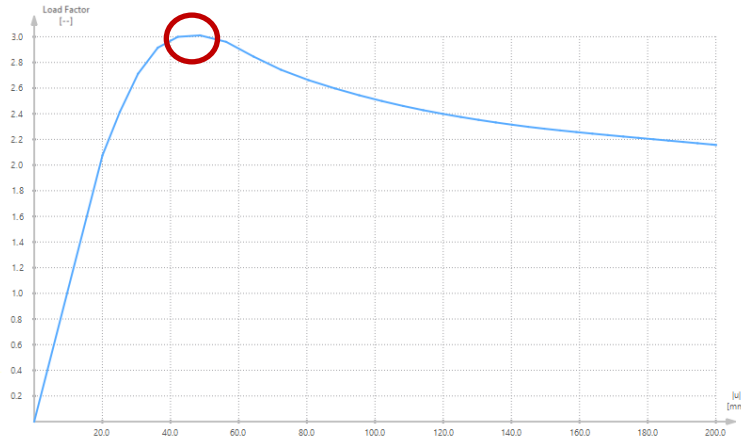
Type of Imperfection	Component	Shape	Magnitude
global, longitudinal stiffener with length a		bow	$\min(a, b) / 400$
local, panel or sub-panel		buckling shape	$\min(a, b) / 200$

Limit states and verification



Maximum load level of the computed load-deformation path $\alpha_{u,1}$

Load level $\alpha_{u,2}$ when reaching largest tolerable principal plastic membrane strain (5%)



$$\min(\alpha_{u,1}, \alpha_{u,2}) = \alpha_u > \alpha_1 \alpha_2$$

Uncertainty of load and resistance models:

Recommendation : γ_{M1} for stability / γ_{M2} for material failure

Uncertainty of numerical model
Recommendation NA: $\alpha_1 = 1,05$

Outlook

Edit Blocks

No. 2 Name PST002

Main Structure Categorization JavaScript

List

- 1 PST001
- 2 PST002
- 3 PST003

Parameters

Geometry

- Panel

Length of buckling panel	a	4.000	m
Width of buckling panel	b	3.000	m
Side ratio	a	1.333	--
- Stiffeners

Number of stiffeners	n	2	
----------------------	---	---	--
- Stiffener 1

Position from top	z1	0.800	m
Position from left	c1	0.200	m
Position from right	d1	0.200	m
Section		1 - 2LLHU(A) L 150x100x10 /310 1 - ...	
Arrangement		Left	
- Stiffener 2

Position from top	z2	1.900	m
Position from left	c2	0.200	m
Position from right	d2	0.200	m
Section		1 - 2LLHU(A) L 150x100x10 /310 1 - ...	
Arrangement		Left	

Material & Thickness

- Panel

Material	1 - Uniform d: 15.0 mm 1 - S235
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Boundary Conditions

Line supports

- Bottom edge

Support	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	(Lines: 1-8,11-14) Hb--
---------	--------------------------	--------------------------	--------------------------	--------------------------	---------------------------
- Top edge

Support	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	(Lines: 1-8,11-14) Hb--
---------	--------------------------	--------------------------	--------------------------	--------------------------	---------------------------
- Left edge

Support	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	(Lines: 1-8,11-14) Hb--
---------	--------------------------	--------------------------	--------------------------	--------------------------	---------------------------
- Right edge

Support	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	(Lines: 1-8,11-14) Hb--
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Nodal supports

- Support 1

Support	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	(Nodes: 1,2,5,6,13,14--)
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- Support 2

Support	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	(Nodes: 1,2,5,6,13,14--)
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Loads

Boundary stresses

- Boundary stresses

Boundary stresses

- Load case

Load case	LC1 - Self weight
-----------	-------------------
- Normal stresses in z-direction

Normal stress Top	$\sigma_{z,1}$	100.000	N/mm ²
Normal stress Bottom	$\sigma_{z,2}$	100.000	N/mm ²
Edge stress ratio	ψ_z	1.000	--
- Shear stress

Shear stress	τ	50.000	N/mm ²
--------------	--------	--------	-------------------
- Normal stress | In z-direction

Normal stress Left	$\sigma_{z,1}$	100.000	N/mm ²
Normal stress Right	$\sigma_{z,2}$	100.000	N/mm ²
Edge stress ratio	ψ_z	1.000	--

OK Cancel Apply





Bibliography

- [1] Eurocode 3: Design of steel structures – Part 1-5: General rules – Plated structural elements; EN 1993-1-5:2006 (E)
- [2] Eurocode 3: Design of steel structures – Part 1-6: Strength and stability of shell structures, EN 1993-1-6:2007 (E)
- [3] Schmidt H.: Beulsicherheitsnachweise für Schalen nach dem neuen Eurocode EN 1993-1-6 – Ein Überblick mit Beispielen aus der Anwendungspraxis, Referat beim 27. Stahlbau-Seminar in Neu-Ulm und Wien, 2005
- [4] ECCS Handbook: Design of steel plated structures with finite elements, 2023



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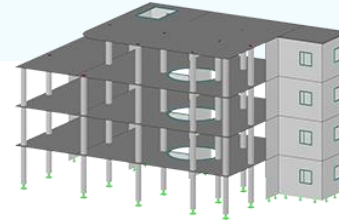
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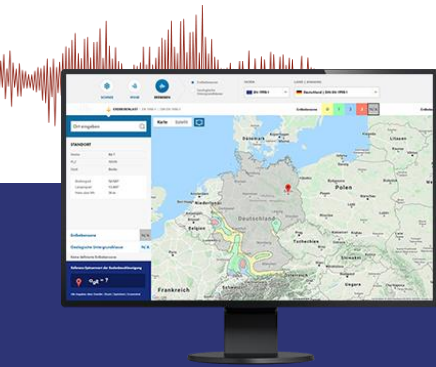


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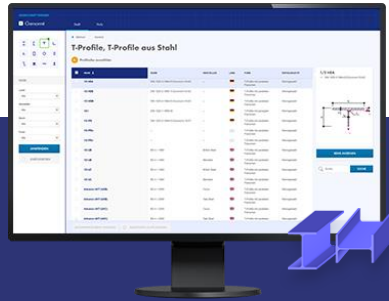
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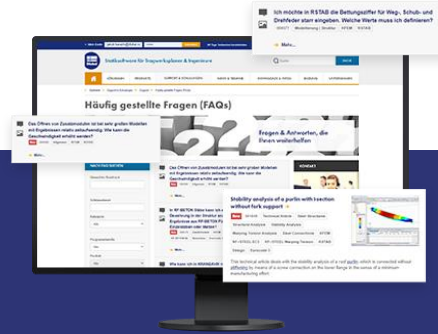
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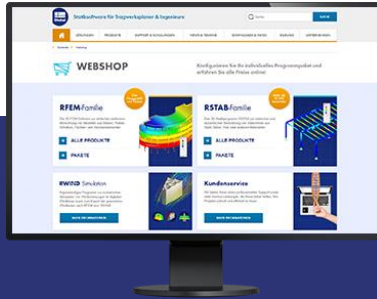
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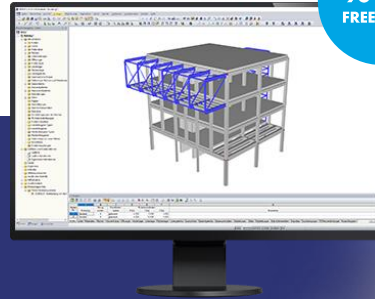
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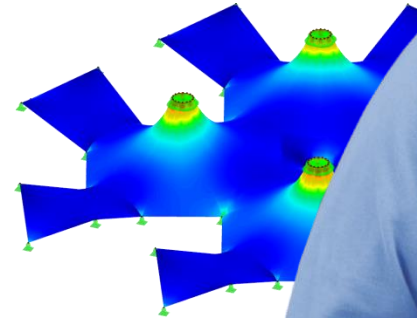
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